Riprap Slope and Outlet Protection BMP 30

Description

Riprap slope and outlet protection is created by an arranged layer or pile of rock placed over the soil surface on slopes and at or below storm drain outfalls or temporary dikes. Riprap used as slope protection protects against erosion and dissipates the energy of runoff or surface water flow. Outlet protection reduces the speed of concentrated stormwater flows, thereby reducing erosion or scouring at stormwater outlets. In addition, outlet protection lowers the potential for downstream erosion. This type of protection can be achieved through a variety of techniques, including stone or riprap outlet structures and armored scour holes installed below the storm drain outlet.

Applications

For slope protection, use riprap or blanketed slopes. Outlet protection should be installed at the outlets of all pipes, culverts, catch basins, sediment basins, ponds, interceptor dikes, and swales or channel sections where the velocity of flow may cause erosion in the receiving channel. Outlet protection should also be used at outlets where the velocity of flow at the design capacity may result in plunge pools (small, permanent pools located at an inlet or outfall). Outlet protection should be installed early during construction activities, but may be added at any time, as necessary.

Limitations

Drainage area – 5 ac.	Maximum slope – 40%
Minimum bedrock depth - N/A	Minimum water table - N/A
NRCS soil type - ABCD	Freeze/thaw – good
Drainage/flood control – no	

The minimum particle size of the rock should be sized for the maximum expected velocity of flow out of the outlet and the soil conditions where the outlet will be located.

Targeted Pollutants

Sediment

Design Parameters

The design of rock outlet protection depends entirely on the location. Pipe outlets at the top of cuts or on slopes steeper than 10%, cannot be protected by rock aprons or riprap sections due to reconcentration of flows and high velocities encountered after the flow leaves the apron.

Tailwater depth: Tailwater depth immediately below the pipe outlet should be determined for the design capacity of the pipe. If the tailwater depth is less than half the diameter of the outlet pipe and the receiving stream is wide enough to accept divergence of the flow, it should be classified as a minimum tailwater condition. If the tailwater depth is greater than half the pipe diameter and the receiving stream will continue to confine the flow, it should be classified as a maximum tailwater condition. Pipes which outlet onto flat areas with no defined channel may be assumed to have a minimum tailwater condition.

Apron Size: The apron length and width should be determined according to the tailwater condition. If the pipe discharges directly into a well-defined channel, the apron should extend across the channel bottom and up the channel banks to an elevation 1 ft above the maximum tailwater depth or to the top of the bank, whichever is less. The upstream end of the apron, adjacent to the pipe should have a width two (2) times the diameter of the outlet pipe, or conform to pipe end section if used.

Bottom Grade: The outlet protection apron should be constructed with no slope along its length. There should be no overfall at the end of the apron. The elevation of the downstream end of the apron should be equal to the elevation of the receiving channel or adjacent ground.

Alignment: The outlet protection apron should be located so that there are no bends in the horizontal alignment.

Materials: The outlet protection may be done using rock riprap, grouted riprap or gabions (BMP 29-Gabions). Riprap size should be based on calculated shear stress. It should be composed of a well-graded mixture of stone size so that 50% of the pieces, by weight, should be larger than the d50 size determined by using the charts. A well-graded mixture as used herein is defined as a mixture composed primarily of larger stone sizes but with a sufficient mixture of other sizes to fill the smaller voids between the stones. The diameter of the largest stone size in such a mixture should be 1.5 times the d50 size. Gabions to be installed in stream banks should be designed and installed according to Rule #9.3 of the *Stream Channel Alterations, Rules and Regulations and Minimum Standards*, Idaho Department of Water Resources, 1978.

Thickness: The minimum thickness of the riprap layer should be 1.5 times the maximum tone diameter for d50 of 15 in. or less; and 1.2 times the maximum tone size for d50 greater than 15 in. Table 30 lists some examples.

Stone Quality: Stone for riprap should consist of field stone or rough unhewn quarry stone. The stone should be hard and angular and of a quality that will not disintegrate on exposure to water or weathering. The specific gravity of the individual stones should be at least 2.5.

Recycled concrete equivalent may be used provided it has a density of at least 150 pounds per cubic ft and does not have any exposed steel or reinforcing bars.

Filter: A filter is a layer of material placed between the riprap and the underlying soil surface to prevent soil movement into and through the riprap. Riprap should have a filter placed under it in all cases.

A filter can be of two general forms: A gravel layer or a plastic filter cloth. The plastic filter cloth can be woven or non-woven monofilament yarns and should meet these base requirements: thickness 10-60 mils, grab strength 90-120 lbs; and should conform to ASTM D-1777 and ASTM D-1682.

Gravel filter blanket, when used, should be designed by comparing particle sizes of the overlying material and the base material. Design criteria are available in any soils or civil engineering reference or from the National Resources Conservation Service.

Construction Guidelines

- The subgrade for the filter, riprap, or gabion should be prepared to the required lines and grades. Any fill required in the subgrade should be compacted to a density of approximately that of the surrounding undisturbed material.
- The rock or gravel should conform to the specified grading limits when installed respectively in the riprap or filter.
- Filter cloth should be protected from punching, cutting, or tearing. Any damage other than an occasional small hole should be repaired by placing another piece of cloth over the damaged part or by completely replacing the cloth. All overlaps whether for repairs or for joining two pieces of cloth should be a minimum of 1 ft.
- Stone for the riprap or gabion outlets may be placed by equipment. Both should be constructed to the full course thickness in one operation and in such a manner as to avoid displacement of underlying materials. The stone for riprap or gabion outlets should be delivered and placed in a manner that will insure that it is reasonably homogenous with the smaller stones and spalls filling the voids between the larger stones. Riprap should be placed in a manner to prevent damage to the filter blanket or filter cloth. Hand placement will be required to the extent necessary to prevent damage to the permanent works.
- Complete construction of the outlet protection before allowing erosive flows to pass through the outlet.

Maintenance

Once a riprap outlet has been installed, the maintenance needs are relatively low. Inspect after heavy storms and high flows for scouring under the outlet and dislodged stones, and repair damage promptly. For dikes, maintain the area upstream of the outlet structure so that accumulated sediments can be removed when they reach a depth of one-third the height of the dike, or 12 in., whichever is less.

Table 30-1. Rock Riprap Sizes and Thickness

Unit shear stress (lb/ft^2)	D_{50} (in.)	d_{max} (in.)	Minimum blanket thickness (in.)
0.67	2	4	6
2.00	6	9	14
3.00	9	14	20
4.00	12	18	27
5.00	15	22	32
6.00	18	27	32
7.80	21	32	38
8.00	24	36	43

Unit shear stress calculated as T = yds
where:
T = shear stress in lb/ft^2
y = unit weight of water, 62.4 lb/ft^3
d = flow depth in ft
s = channel gradient in ft/ft

Design Procedure and Examples

- Investigate the downstream channel to assure that non-erosive velocities can be maintained.
- Determine the tailwater condition at the outlet to establish which curve to use.
- Enter the appropriate chart with the depth of flow and discharge velocity to determine the riprap size and apron length required. It is noted that references to pipe diameter in the charts are based on full flow. For other than full pipe flow, the parameters of depth of flow and velocity should be used.
- Calculate apron width at the downstream end if a flared section is to be employed.

Example 1: *Pipe Flow (full) with discharge to unconfined section*
A circular conduit is flowing full:
Q = 280 cfs, diam. = 66 in., tailwater (surface) is 2 ft above pipe invert, (minimum tailwater condition)
Read d_{50} = 1.2 ft, and apron length 38 ft
Apron width = diam. + L_a = 5.5 + 38 = 43.5 ft

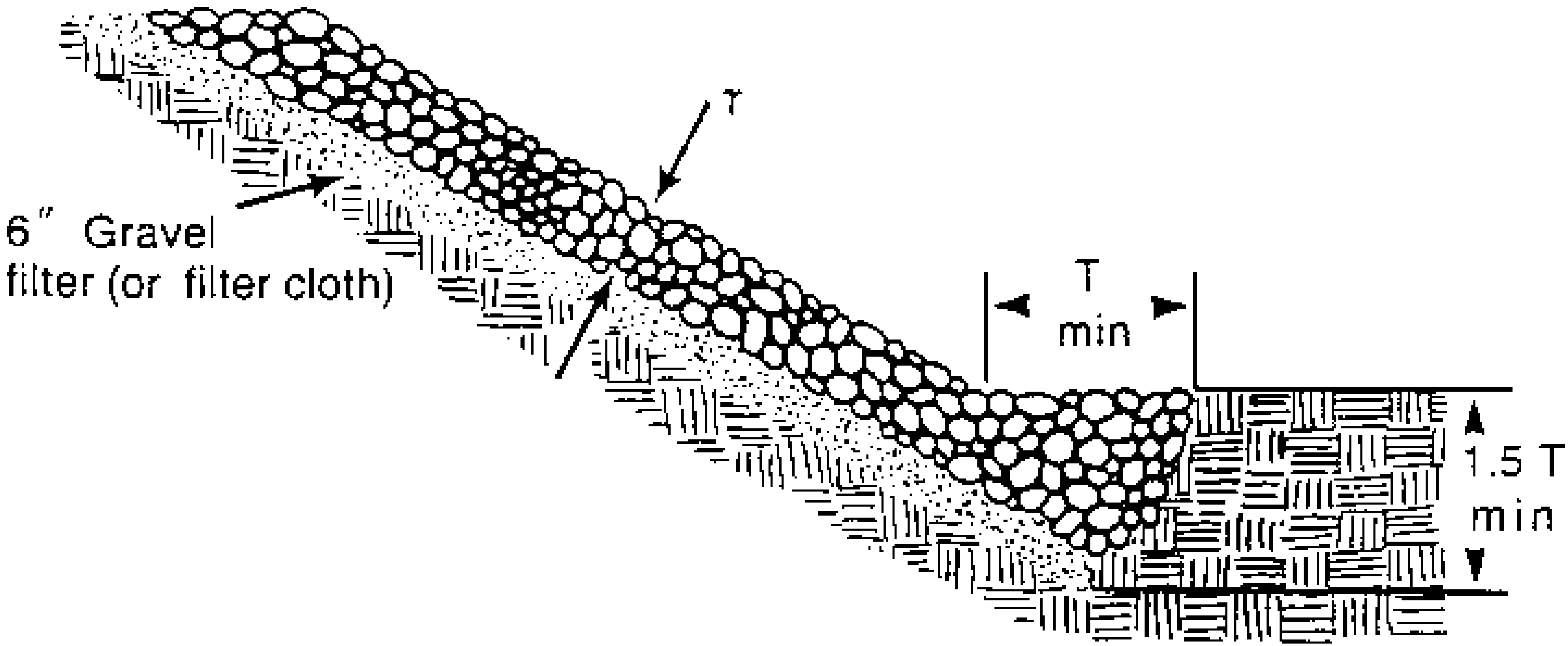
Example 2: *Box Flow (partial) with high tailwater*
A box conduit discharging under partial flow conditions. A concrete box 5.5 x 10 ft is flowing 5.0 ft deep, Q = 600 cfs and tailwater surface is 5 ft above invert (Max. tailwater condition).

$V = \frac{Q}{A} = \frac{600}{5x10} = 12$ fps

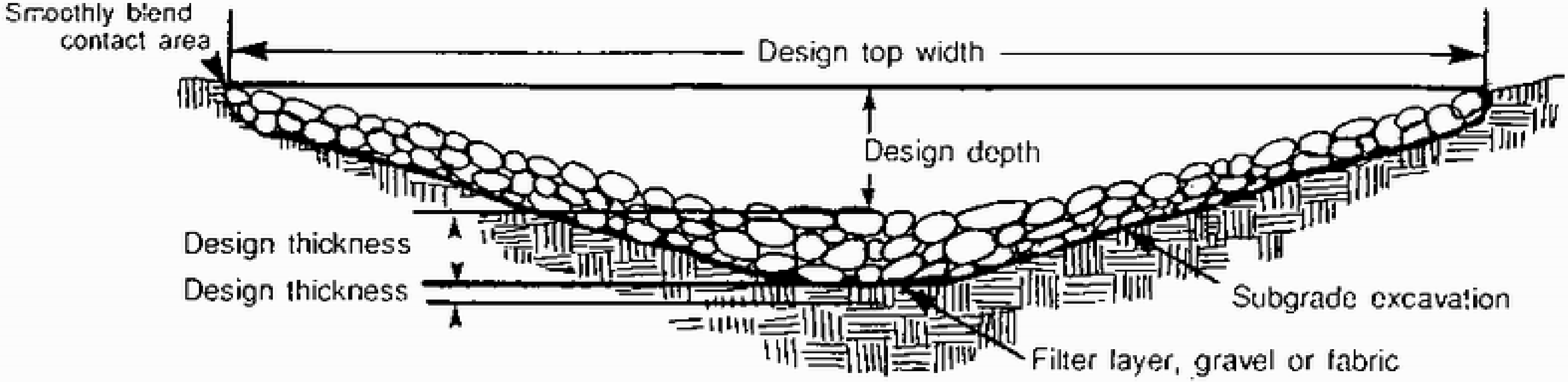
At the intersection of the curve d = 60 in. and V = 12 fps, read $d_{50} = 0.4$ ft
Then reading to the d = 60 in. curve, read apron length = 40 ft
Apron width, W = conduit width + 0.04 L_a = 10+(0.4) (40) = 26 ft

Example 3: *Open Channel Flow with Discharge to Unconfined Section*
A trapezoidal concrete channel 5 ft wide with 2:1 side slopes is flowing 2 ft deep, Q = 180 cfs (velocity = 10 fps) and the tailwater surface downstream is 0.8 ft (minimum tailwater condition).
At intersection of the curve d-24 ft and V = 10 fps, read $d_{50} = 0.7$ ft
Then reading up to the d = 24 in. curve, read apron length = 22 ft
Apron width, W = bottom of width of channel + L_a = 5 + 22 = 27 ft

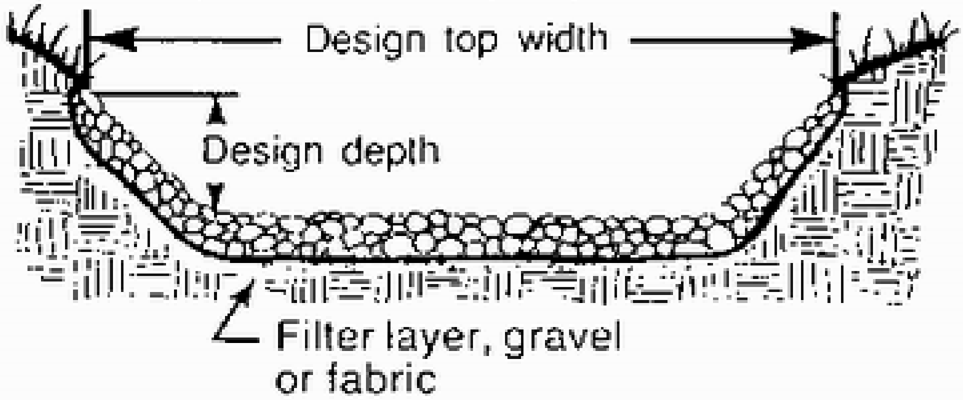
Example 4: *Pipe flow (partial) with discharge to a confined section*
A 48 in. pipe is discharging with a depth of 3 ft, Q = 100 cfs, and discharge velocity of 10 fps (established from partial flow analysis) to a confined trapezoidal channel with a 2 ft bottom, 2:1 side slopes, n = .04, and grade of 0.6%.
Calculation of the downstream channel (by Manning's Equation) indicates a normal depth of 3.1 ft and normal velocity of 3.9 fps.
Since the receiving channel is confined, the maximum tailwater condition controls.
At the intersection of d = 36 in. and v = 10 fps, Read $d_{50} = 0.3$ ft
Reading up to the d = 36 in. curve, read apron length = 30 ft
Since the maximum flow depth in this reach is 3.1 ft that is the minimum depth of riprap to be maintained for the entire length.


T
6" Gravel
filter (or filter cloth)
T
min
1.5 T
min

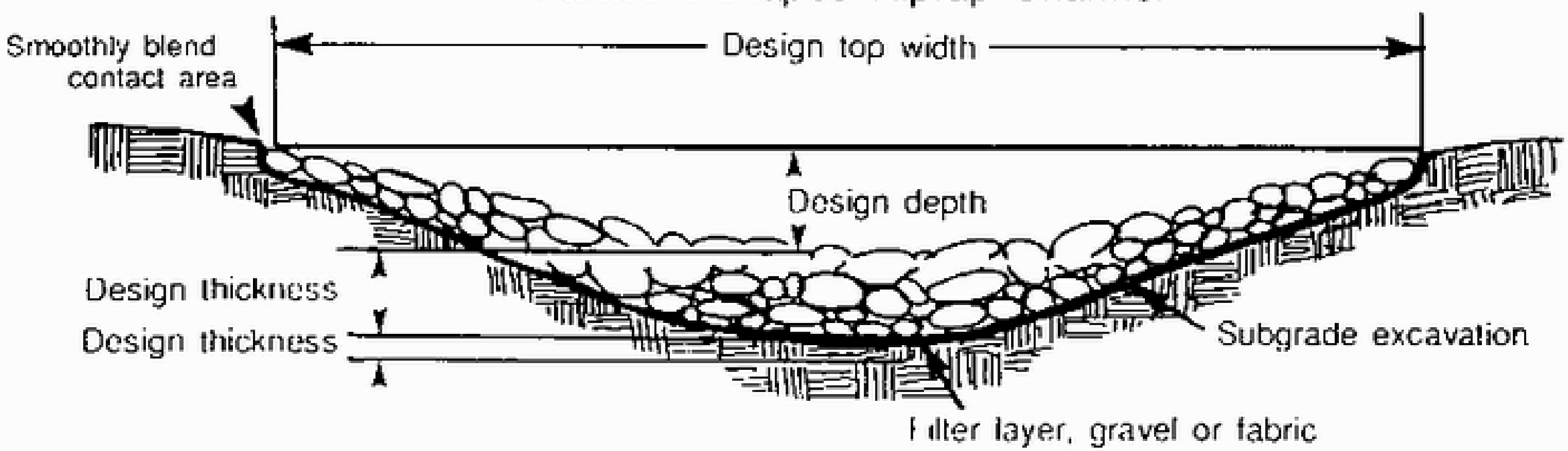
V-shaped Riprap Channel

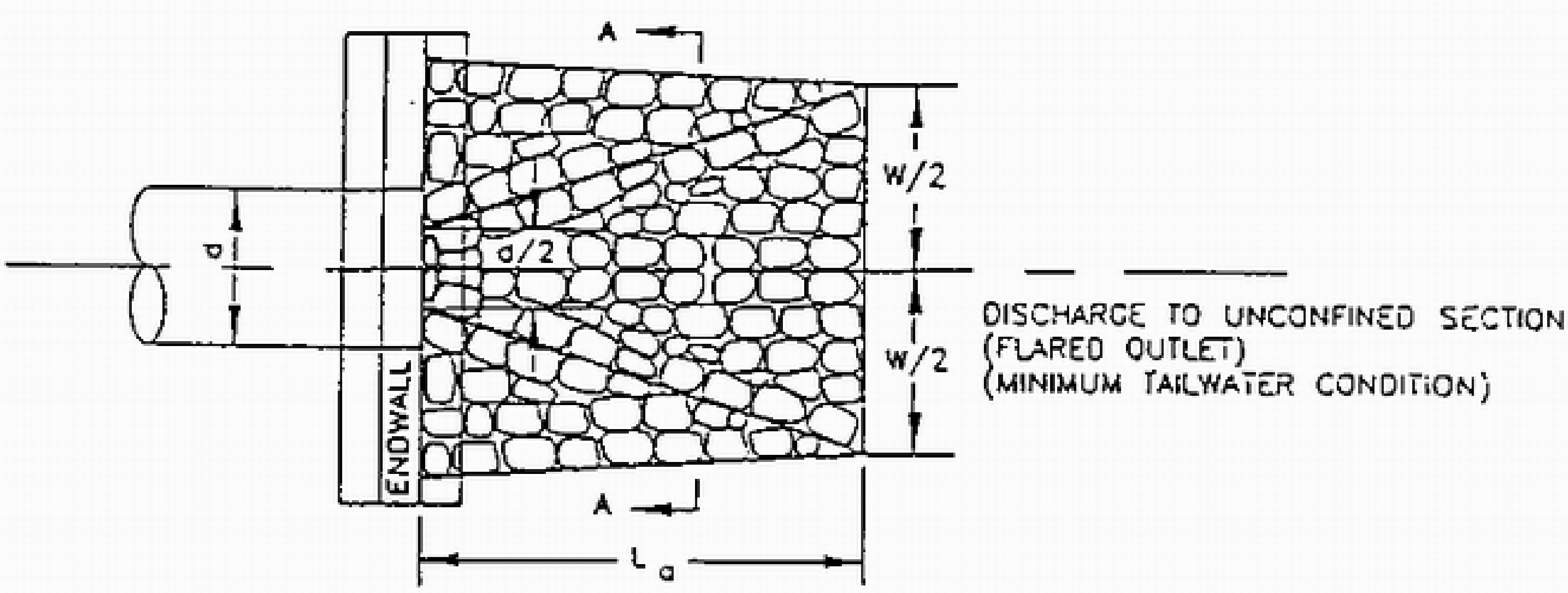


Trapezoidal Riprap Channel

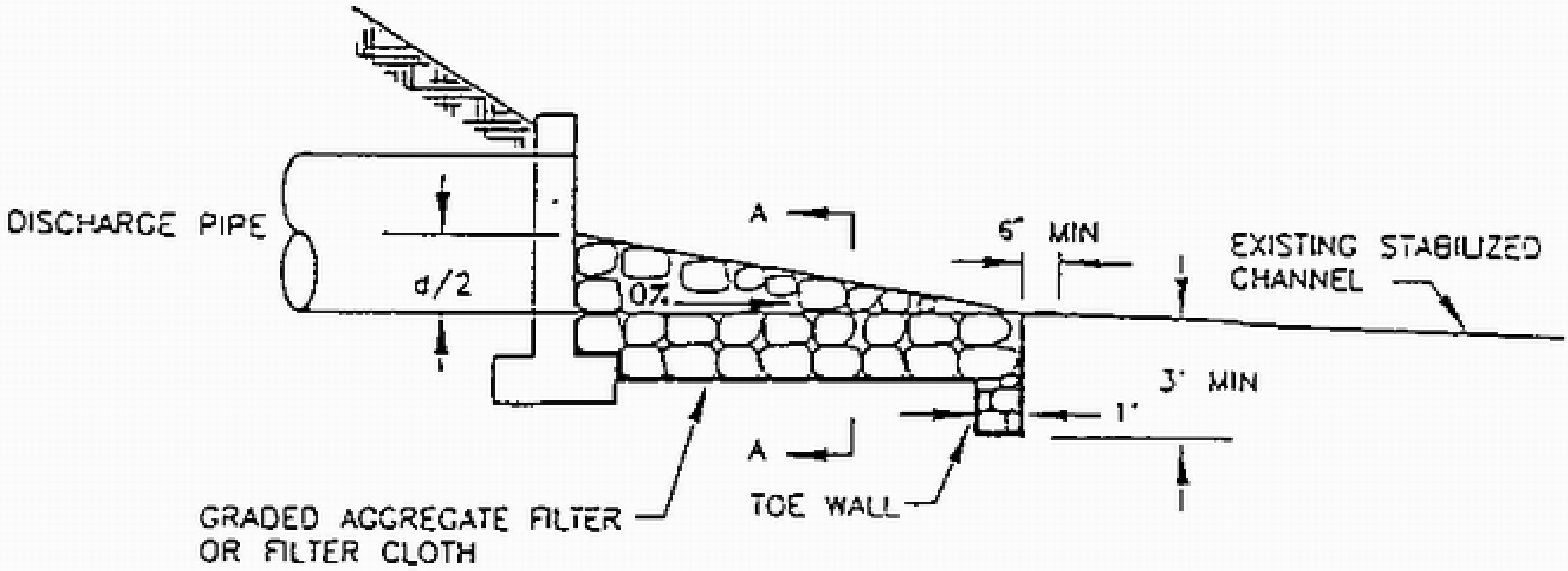


Parabolic-shaped Riprap Channel

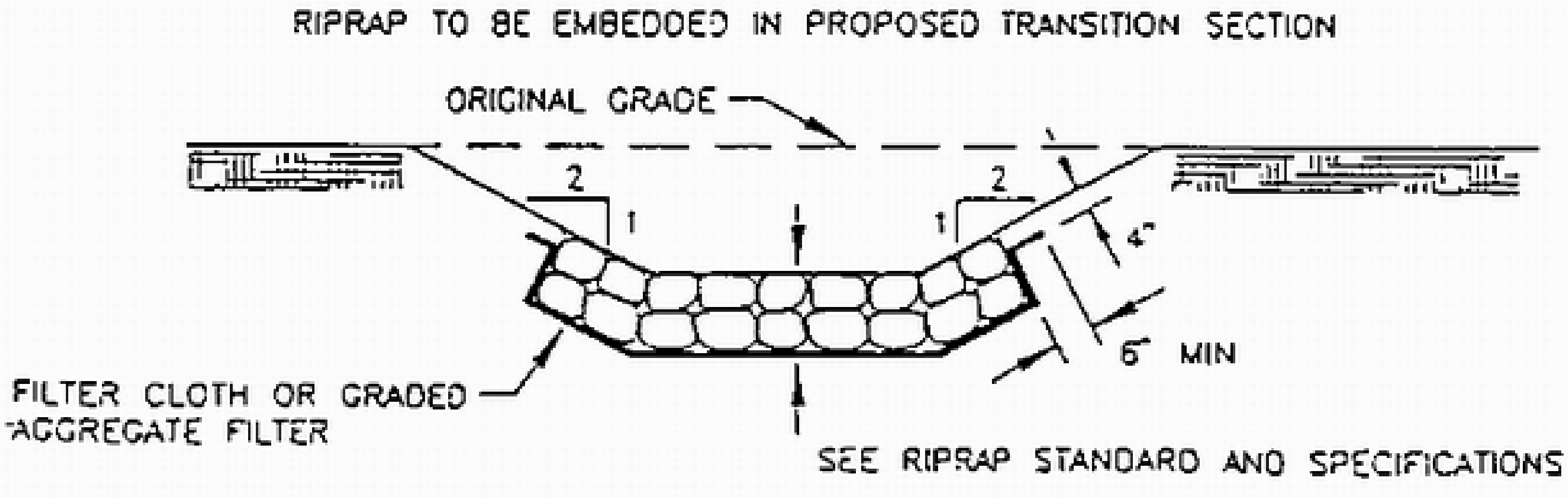




PLAN VIEW

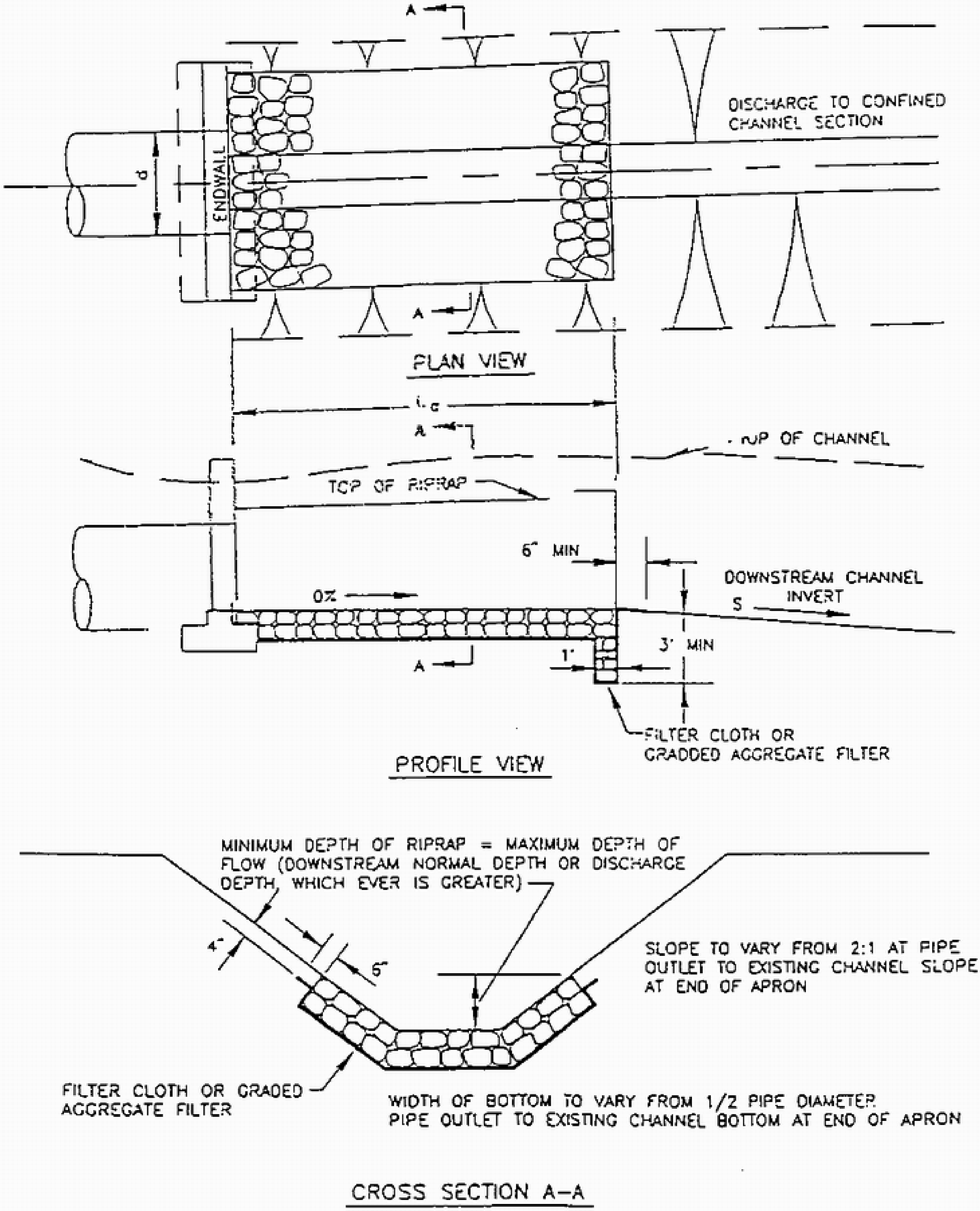


PROFILE VIEW



CROSS SECTION A–A

U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE	RIPRAP OUTLET PROTECTION – I	STANDARD DRAWING
TOOTHMAN–ORTON ENGINEERING COMANY BOISE, IDAHO McCALL, IDAHO		ROP–I



U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE	RIPRAP OUTLET PROTECTION - II	STANDARD DRAWING
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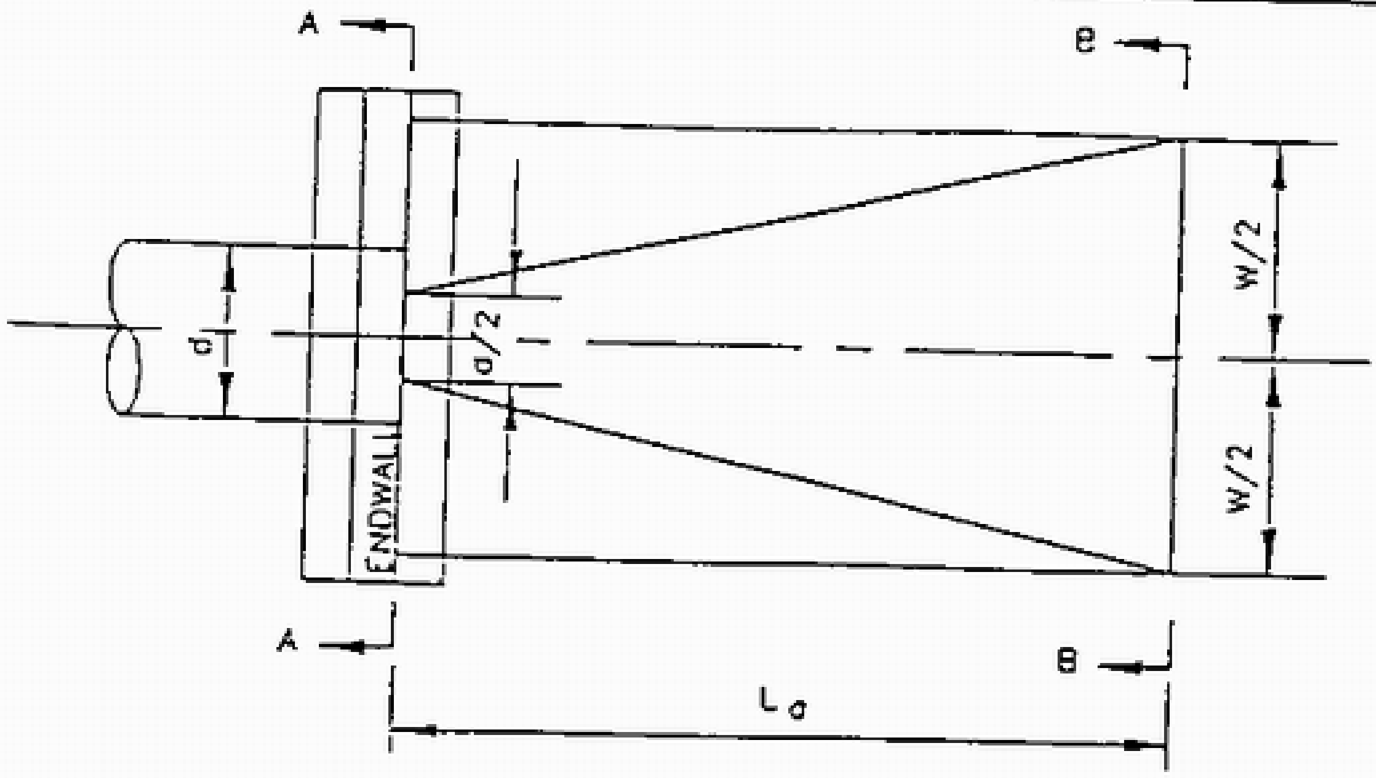
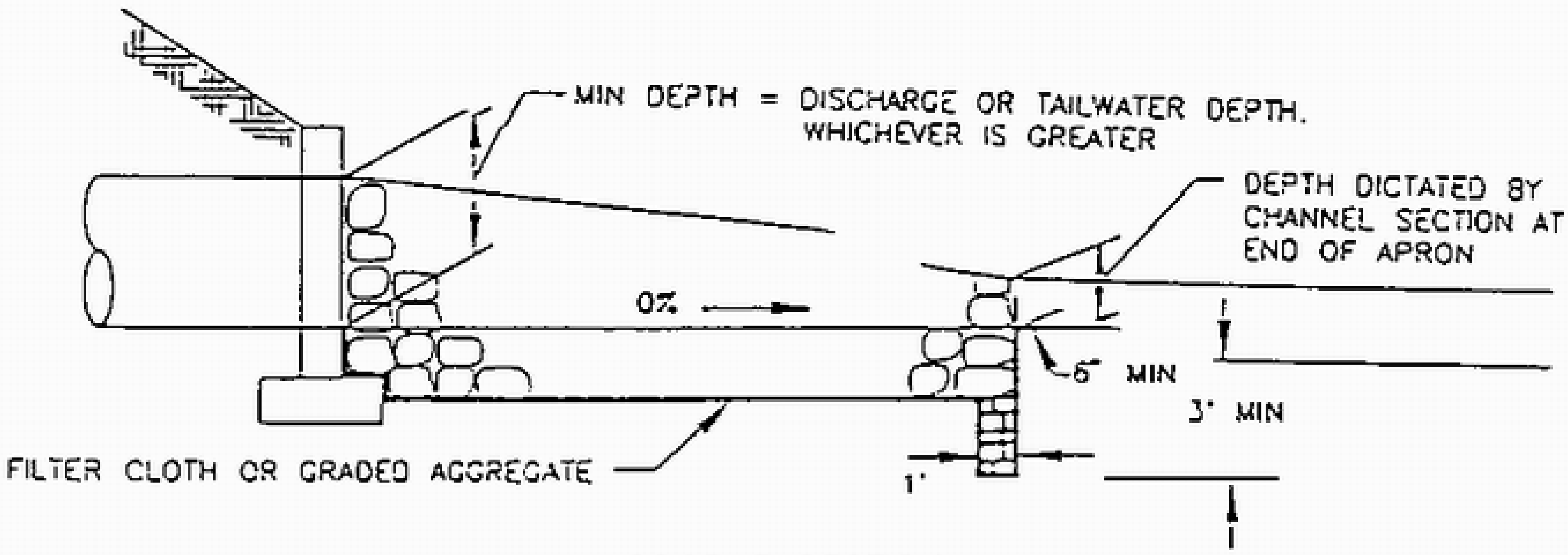


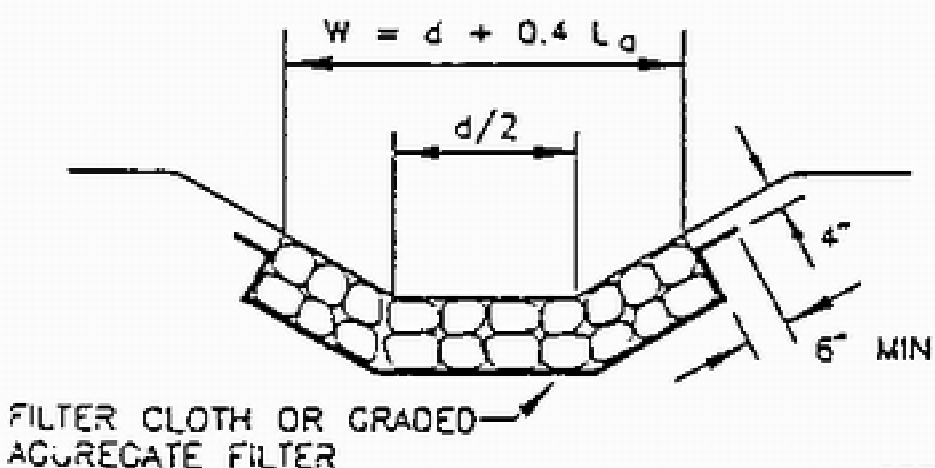
DISCHARGE TO SEMI CONFINED SECTION (MAXIMUN TAILWATER CONDITION)

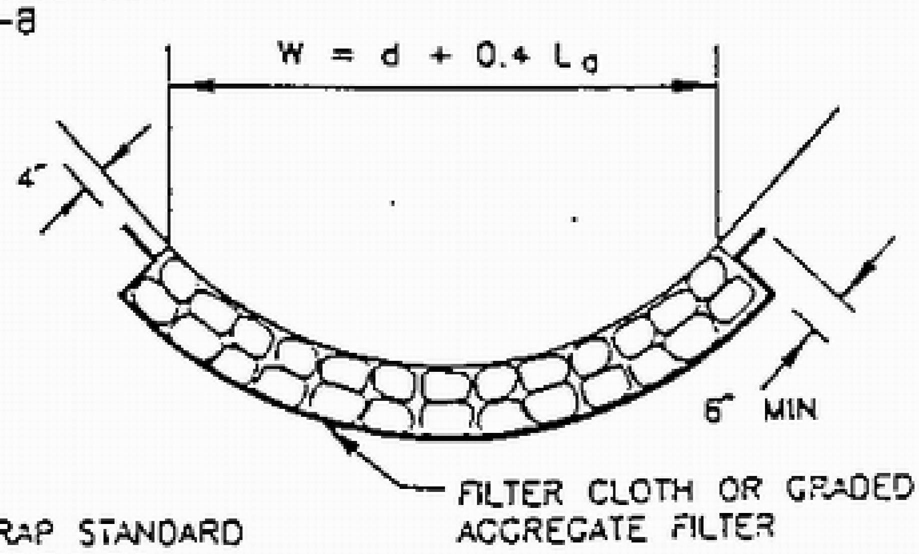
PLAN VIEW



PROFILE SECTION

CHANNEL SECTION TO VARY FROM A-A TO B-B





SEE RIPRAP STANDARD AND SPECIFICATION

CROSS SECTION A-A

CROSS SECTION B-B

U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE	RIPRAP OUTLET PROTECTION - III	STANDARD DRAWING
TOOTHMAN-ORTON ENGINEERING COMANY BOISE, IDAHO McCALL, IDAHO		ROP-III